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Evaluation of Rate of Deterioration in Various Concrete

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ABSTRACT: The extensive use of concrete as a structural material has lead to the demand to fully understand the effect of fire on concrete. With the evolution of High Strength Concrete (HSC), questions have arisen as to whether this type of concrete can still be effective as a fire proofing material. As concrete is exposed to elevated temperatures in fire, its mechanical properties such as strength and modulus of elasticity may be decreased remarkably and results in undesirable fractures and spalling. Although a great deal of research has been conducted on the fire resistance of concrete, since HSC is a relatively new type of concrete, knowledge about the performance of HSC that too with blended cement subjected to fire is limited in comparison with Normal Strength Concrete (NSC).

A recent review of fire performance of HSC identifies that HSC differs from NSC with respect to relative strength loss and the occurrence of explosive spalling in HSC. An attempt has been made in this work to study the effect of elevated temperatures (up to 1000°C) on compressive strength of HSC of M80 grade using ordinary Portland cement. High strength concrete specimens were exposed to different temperatures of 200°C to 1000°C duration of 1hour. After exposing the specimens to elevated temperatures, they were tested for their strength in hot state. Then effect of elevated temperatures on strength of HSC was assessed.

KEYWORDS: High strength concrete, conventional concrete, temperature, split tensile strength, flexural strength.

I. INTRODUCTION

The twentieth century has brought about much technological advancement in all areas of life, including construction. High-strength concrete (HSC) is the current state-of-the-art in concrete construction. HSC is a truly efficient building material continually being pushed to ever-greater strengths and newer used by leading professionals. And it is poised for even further development in the 21st century and beyond. High-strength concrete as we know it today in the ready mix and precast industries is the result of ongoing research and development to optimize concrete ingredients, proportioning, mixing, placing, and quality control methods.

High strength concrete has a low water cement ratio and higher compressive strength than conventional concrete. To achieve higher workability, super plasticizers are added. The higher compressive strength is the result of admixtures added during mixing.

A USE OF HIGH STRENGTH CONCRETE

Strength: Use of high strength concrete can make concrete construction a viable economic alternative to steel frames in certain high rise buildings. In such cases normal strength requires excessively large column sizes.

Cost: Use of high strength concrete and consequent reduction in column size can dramatically increase floor space.

Modulus: The desire to reduce sway and increase stiffness of buildings can be a reason for use of the higher modulus of



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some high strength concretes.

Durability: Reduced permeability of higher strength concretes can help deter chemical attack, and improve abrasion resistance.

II. OBJECTIVE

The main objective of this thesis is to arrive at the mix ratio M80 for high strength concrete and M25 for normal concrete. To compare and analyse the rate of deterioration of high strength and normal concrete. Usage of high strength concrete has been increasing day by day in construction industry. Therefore investigations on its deterioration will be of great use, keeping in view of the life of the structure.

III. PROPERTIES OF MATERIALS

The properties of various materials used in both conventional and high strength concrete are tabulated below

S. No.	Description	Parameters
1	Grade of concrete	M25
2	Cement	OPC 53 grade IS 269-1976
3	Fine aggregate	Sand confirming to zone II of IS 383-1978
4	Coarse Aggregate	10-12mm nominal size
5	Type & shape of C.A	Angular
6	W/c ratio	0.53

Table 1 Properties of various materials used in conventional concrete

Table 2 Properties of various materials used in high strength concrete

S. No.	Description	Parameters
1	Grade of concrete	M80
2	Cement	OPC 53 grade IS 269-1976
3	Fine aggregate	Sand confirming to zone II of IS 383-1978
4	Coarse Aggregate	10-12mm nominal size
5	Type & shape of C.A	Angular
6	W/c ratio	0.31



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IV. TESTS ON MATERIALS

Various test are conducted on different materials like cement, sand, coarse aggregate and the values are tabulated below:

Table 3 Test on various materials						
S.NO	TEST	RESULT				
1	Normal consistency of the cement	29 %				
2	Initial setting time of cement	25 min				
3	Specific gravity of cement	3.60				
4	Fineness of cement sample	98.27%				
5	Specific gravity of sand	2.47				
6	Specific gravity of coarse aggregate	2.91				

V. DESIGN MIX

Mix design for M25 and M80 concrete is derived and the mix ratios are tabulated below

Water cement ratio	Cement content	Fine aggregate content	Coarse aggregate content
211.72 lit	355.6 kg	643.74 kg	1131.87kg
0.53	1	1.810	3.183

Table 5 Mix ratio for M80 concrete

Cement content	Fine aggregate content	Coarse aggregate content	Water cement	Super plasticizer
504.21 kg	683.24 kg	1108.13 kg	141.611	4.66811
1	1.35	2.19	0.31	0.8



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VI. RESULTS

A Compressive strength

Compressive strength of a concrete is a measure of its ability to resist static load, which tends to crush it. Most common test on hardened concrete is compressive strength test Compressive strength is one of the important properties of concrete. Conventional Concrete cubes of size 100x100x100mm were casted. After 24 hours the specimen were demoulded and subjected to water curing. After 7, 14 & 28 days curing three cubes were taken and allowed to dry and tested in compressive strength testing machine.

Age of (Days)	curing	Conventional concrete M25 (N/mm ²)	High strength concrete M80 (N/mm ²)
7		17.83	41.2
14		19.30	50.65
28		21.26	63.25

Table 6 Compressive strength of conventional and high strength concrete

B. Tensile strength

Tensile strength of a concrete is a measure of its ability to resist forces, which stretch or bend it. Specimens of size 200x100mm were casted. After 24 hours the specimen were demoulded and subjected to water curing. After 7, 14 & 28 days curing three cubes were taken and allowed to dry and tested in split tensile strength testing machine.

Age of curing (Days) Conventional concrete M25 (N/mm²) High strength concrete M80 (N/mm²)						
7		2.9	5.75			
14		3.25	6.1			
28		3.6	6.45			

Table 7 Split tensile strength of conventional and high strength concrete

C Flexural strength

The specimen is placed in universal testing machine and the load is to be applied without shock until the specimen fails and the maximum load applied to the specimen during the test is to be recorded.

Table 8 Flexural strength of conventional and high strength concrete

Age of (Days)	curing	Conventional concrete M25 (N/mm ²)	High strength concrete M80 (N/mm ²)
7		3.45	6.8
14		3.7	7.15
28		4.16	7.35

D. COMPRESSIVE STRENGTH OF DETERIORTED CONCRETE AND PERCENTAGE DETERIORATION Cured specimens of 7, 14 and 28 days were kept in oven at different temperatures such as 200, 400, 600, 800,

1000°c. Specimens are heated for 1 hour, after it was cooled in room temperature for some time. Compressive strength test on the specimens were carried out.



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Table 9 Compressive strength of conventional and high strength concrete after deterioration

Temperature	7 th day (N/mm ²)		14 th day (N/mm ²)		28 th day (N/mm ²)	
	M25	M80	M25	M80	M25	M80
200	16.5	41	18.7	50.5	20	63.2
400	15.4	39.5	17.2	49	18.5	61.5
600	13.9	36	15.6	45.1	16.7	56.5
800	11.6	32.2	13.9	40.5	14.3	50.85
1000	9.5	27.5	10.8	33.9	11.6	43.26

Table 10 Percentage Deterioration in compressive strength of conventional and high strength concrete

concrete							
Temperature	7 th day		14 th day		28 th day		
	(%)		(%)		(%)		
	M25	M80	M25	M80	M25	M80	
200	92.5	99.5	96.9	99.7	94.07	99.92	
400	86.3	95.87	89.11	96.74	87.01	97.23	
600	77.95	87.3	80.82	89.04	78.55	89.32	
800	65.05	78.15	72.02	80	67.26	80.47	
1000	53.28	66.74	55.95	67	54.56	68.4	



Fig.1 Line graph for percentage deterioration for compressive strength of concrete

E. SPLIT TENSILE STRENGTH OF DETERIORTED CONCRETE AND PERCENTAGE DETERIORATION Cured specimens of 7, 14 and 28 days were kept in oven at different temperatures such as 200, 400, 600, 800, 1000°c. Specimens are heated for 1 hour, after it was cooled in room temperature for some time. Split tensile strength test on the specimens were carried out.



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Table 11 Split tensile strength of conventional and high strength concrete after deterioration

Temperature	7 ^m day (N/mm ²)		14 th day (N/mm ²)		28 th day (N/mm ²)	
	M25	M80	M25	M80	M25	M80
200	2.85	5.7	3.2	6.1	3.56	6.45
400	2.65	5.4	3	5.75	3.3	6.05
600	2.36	4.85	2.65	5.26	3	5.45
800	2.05	4.19	2.29	4.5	2.66	4.89
1000	1.66	3.45	1.8	3.75	2.12	4.05

Table 17 Percentage	Deterioration in S	nlit tencile strength /	of Conventional and	d high strength concrete
Table 12 I cicchage	Detter for attom in 5	phi tensne su engui v		a mgn strength concrete.

Temperature	7 th day		14 th day		28 th day	
	(%)		(%)		(%)	
	M25	M80	M25	M80	M25	M80
200	98.27	99.13	98.46	100	98.46	100
400	91.37	93.91	92.3	94.26	91.66	93.8
600	81.37	84.34	81.53	86.22	83.33	85
800	70.68	72.8	70	73.77	73.88	75.8
1000	57.24	60	55.38	61.47	58.8	62.8



Fig. 2 Line graph for percentage deterioration in split tensile strength of concrete

F FLEXURAL STRENGTH OF DETERIORTED CONCRETE AND PERCENTAGE DETERIORATION Cured specimens of 7, 14 and 28 days were kept in oven at different temperatures such as 200, 400, 600, 800, 1000°c. Specimens are heated for 1 hour, after it was cooled in room temperature for some time. Flexural strength test on the specimens were carried out.



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Table 13 Flexural strength of conventional and high strength concrete after deterioration

Temperature	7 ^m day (N/mm ²)		14 th day (N/mm ²)		28 th day (N/mm ²)	
	M25	M80	M25	M80	M25	M80
200	3.4	6.8	3.65	7.15	4.1	7.35
400	3.15	6.35	3.4	6.75	3.85	6.95
600	2.76	5.8	3	6.16	3.36	6.29
800	2.23	4.96	2.58	5.25	2.9	5.48
1000	1.86	4.38	2.05	4.68	2.3	4.92

Table 14 Percentage Deterioration in flexural strength of conventional and high strength concrete

Temperature	7 th day		14 th day		28 th day	
-	(%)		(%)		(%)	
	M25	M80	M25	M80	M25	M80
200	98.55	100	98.9	100	98.55	100
400	91.3	93.38	91.89	94.4	92.5	94.55
600	80	88.29	81.08	86	80.76	85.5
800	64.63	73.1	67.56	73.42	69.7	74.55
1000	54	64.4	55.4	65.45	55.28	64.25



Fig 3 Line graph for percentage deterioration for flexural strength of concrete

VII. CONCLUSION

From the experiment results, the following conclusions were drawn

- Upto 200°C high strength concrete maintained its original strength while normal concrete lost its strength marginally.
- > In the range of 400 to 800°C, both high strength and normal strength concrete lost most of their



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original strength, especially at temperatures above 600°C, there is a rapid deterioration in strength of concrete.

- > Therefore, it is only within the range of 400 to 800°C that research efforts can be carried out to reduce the strength loss and improve the thermal behavior of concrete.
- > Upto 200°C high strength concrete is used where temperature resistancy is required.

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